# ECI 141: ENGINEERING HYDRAULICS

# MID-TERM EXAMINATION

**FALL 2005** 



NAME ....

# ANSWER ALL QUESTIONS.

For water, take  $\rho = 10^3 \text{ kg/}m^3$  (=1.94 slug/ $ft^3$ ) and  $\mu = 10^{-3} \text{ kg/m.s}$  (=2.09 × 10<sup>-5</sup> slug/ft.s).

A cast-iron horizontal pipe is used to carry water at a mean velocity of 6 ft/s.

The pipe has diameter D = 6 inches, length L = 200 ft and has roughness length  $\epsilon = 0.0004$  ft.

D= 6in. 11 = 0.5 ft

Calculate:

1. the head loss,  $\mathcal{E} = 0.0004 \text{ ft} \qquad \mathcal{E} = 0.0004 \text{ ft} = 8.10^{-4}$ 2. the pressure drop,  $V = \int_{0.5}^{0.5} f_{0.5} = 8.10^{-4}$ 3. the input power to the pump required to convey the water (assume pump efficiency of 70%).

Re= PN = (1.94) (6) (0.5) = 2.8.105 (turbulent)

En= 8.10-1= 0.0008

M-C with \$5 = 0.0008 and fe = 2.8.005 -> f≈ 0.0185

$$h_f = f \int \frac{L}{Z_g} = (0.0185) \frac{(200)}{(0.5)} \frac{(6^2)}{(2.32.2)} = 4.14 f \rightarrow h_f = 4.14 f \uparrow 10$$

Δp = hf -> Δp= eghp = (1.94)(32.2)(4.14) = 258.62 shy AF. AF → [Δp = 258,62 sty 45]

3.  $y = 0.7 = \frac{power in}{power est} \rightarrow power in = 0.7 power at$ Q= 70° V = 7(.5°)(6)=1.18 43 power = eghpma Q

$$\Rightarrow \text{ person} = \underbrace{(1.94)(32.2)(4.14)(1.18)}_{0.7} = 435.96 \text{ sky. } \underbrace{4.44}_{35.96} \underbrace{4.44}_$$

2 - France = 435.96 shy H2 53

1.

Oil ( $\rho = 950 \text{ kg/m}^3$  and  $\mu = 0.019 \text{ kg/m.s}$ ) flows through a pipe of diameter D = 0.3 m and length L = 100 m. The pipe roughness length ratio  $\epsilon/D =$ 0.0002.

ED = 0.000Z

The head loss due to friction is measured at 8 m.

D=0.3

1 = 100 m

Find the average velocity and the flow rate.

P = 950 Kg = 3

You may use either the Direct Solution or the Iterative Solution.

M=0.019 Kg

$$B = \frac{140 (20)}{L} = \frac{(8)(0.3)(2.9.81)}{(100)} = 0.471 \frac{m^2}{52}$$

hc = 8m

$$h_{f} = \frac{8 \text{ AL}}{g \pi^{2} b^{3}} Q^{2} \Rightarrow \underbrace{8(9.8)(\pi^{2})(0.3^{5})}_{8(100)} = f Q^{2} \Rightarrow 0.00235 = f Q^{2}$$

$$Q = Iq D^{2} V \Rightarrow 0.00235 = f \left(\frac{\pi^{2}}{16}\right)(0.3^{4}) V^{2}$$

$$\Rightarrow \oint = \frac{0.47}{V^2} \quad (1)$$

(levative) 
$$\Rightarrow$$
 assume  $f = 0.03$ .  
 $\Rightarrow w = 0.03$ ,  $e_b = 0.0002$   $\Rightarrow h_e = f = \frac{1}{5} \frac{v^2}{5}$   
 $\Rightarrow V = h_e D (2a) = \sqrt{\frac{8}{0.03}(100)} = 3.96 \% \Rightarrow Re = \frac{910}{5} = 5.9.10 \%$ 

$$Q = \sqrt{q} V = \sqrt{q} (0.3^2) (4.79) = 0.339 \frac{m^3}{5}$$

$$Q = \sqrt{Q} = 0.339 \frac{m^3}{5}$$

A pump is required to transport water between two reservoirs. The distance between the free surface levels in the two reservoirs is 120 ft.

The reservoirs are connected with a cast iron pipe of length L = 2000 ft, diameter D = 6 inch and roughness height ratio  $\epsilon/D = 0.0017$ .

x 200

A number of minor losses are present. The sum of all the minor loss coefficients is K=2.5.

Calculate the power of the pump needed to pump water at a rate of 3  $ft^3/s$  (assume 75% efficiency).

(assume 75% efficiency).

L = 2000 ft

$$Q = 3$$
 $\frac{1}{3}$ 
 $\frac{1}{3}$ 

Water flows along a rectangular channel at a depth of 1.3 m. The channel's width is 5.5 m, its slope is 0.001, and has a Manning's n of 0.024.

A rectangular bump is placed on the bed of this channel some distance after uniform flow is established.

### Calculate:

- 1. the height of this bump required to produce **critical** flow on the crest,
- 2. the maximum depression in the water-surface level above the bump.



$$Q = \frac{C_{m}}{N} A R^{\frac{3}{3}} S_{0}^{\frac{1}{2}} = \frac{1}{0.024} (7.15) (0.883)^{\frac{3}{3}} (0.001)^{\frac{1}{2}} = 8.67 \text{ m}^{\frac{3}{5}}$$

$$A_{c} = \left(\frac{b_{c} Q^{2}}{g}\right)^{\frac{1}{3}} = \left(\frac{6.5}{(9.8)}\right)^{\frac{1}{3}} = 3.48 \text{ m}^{\frac{3}{2}}$$

$$Q = Q \rightarrow \frac{8.67}{5.5} = 1.576 \rightarrow 9 = 1.576 \text{ m}^{\frac{3}{5}} \stackrel{1}{1}$$

$$Q = A_{c} V_{c} = A_{c} V_{c} \rightarrow V_{c} = \frac{Q}{A_{c}} = 2.49 \text{ m}^{\frac{3}{5}} \rightarrow V_{c} = 1.21 \text{ m}^{\frac{3}{5}}$$

$$A_{c} = y_{c} \rightarrow y_{c} = \frac{3.48 \text{ m}^{2}}{5.5 \text{ m}} = 0.63 \text{ m}^{\frac{3}{3}}$$

$$E_{c} = E_{2} + \Delta h \rightarrow y_{c} + \frac{V_{c}^{2}}{2y} = y_{c} + \frac{V_{c}^{2}}{2y} + \Delta h \rightarrow 1.375 = 0.946 \text{ m} + \Delta h$$

$$\rightarrow \Delta h = 0.429 \text{ m}$$

2.

$$y_1 = \Delta h + y_c + w_s \Rightarrow w_s = y_1 - \Delta h - y_c = 1.3 - 0.429 - 0.63 = .241$$

depression in water surface  $\leq 0.241 \, \text{m}$ 
 $t = 3$ 

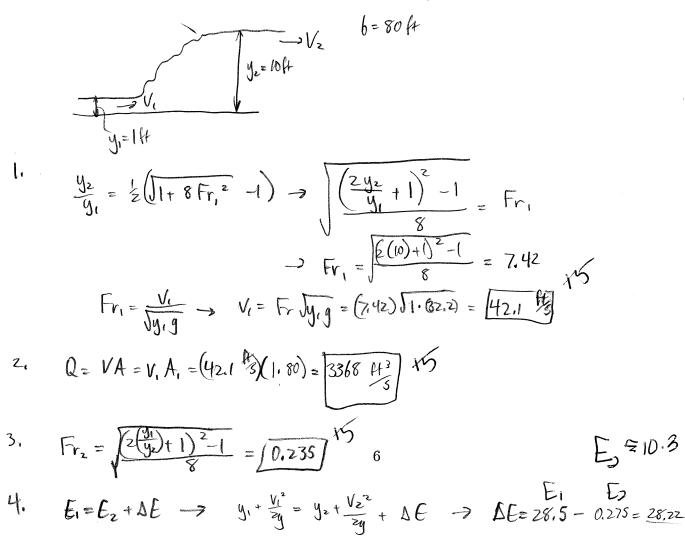
A hydraulic jump is induced in an 80 ft wide channel.

The water depths on either side of the jump are 1 ft and 10 ft.

x 1/20

Calculate:

- 1. The velocity of the faster moving flow,
- 2. The flow rate,
- 3. The Froude number of the sub-critical flow,
- 4. The flow energy dissipated in the hydraulic jump (expressed as percentage of the energy prior to the jump).



x7 % dissipated = DE = 28,22,100 = 99 %